

APPENDIX H FEMA FORM 81 – 89B

This appendix presents FEMA FORM 81 - 89B with geotechnical information provided based on the results of our geotechnical evaluation.

U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY RIVERINE STRUCTURES FORM

O.M.B No. 1660-0016 Expires: 12/31/2010

PAPERWORK REDUCTION ACT

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Flooding Source:		
Note: Fill out one form for each flooding source studied		
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A GENERAL

			A. GENERAL		
Comp	plete the appropriate secti	ion(s) for each Structure	listed below:		
	Channelization	complete Section C complete Section D	required)		
Desc	ription Of Structure				
1.	Name of Structure:	Dad's Point	Levee		
	Type (check one):	☐ Channelization	☐ Bridge/Culvert	Levee/Floodwall	☐ Dam/Basin
	Location of Structure:	STOCKTON,	California		
	Downstream Limit/Cros	-			
	Upstream Limit/Cross S	Section:			
2.	Name of Structure:				
	Type (check one):	☐ Channelization	☐ Bridge/Culvert	☐ Levee/Floodwall	☐ Dam/Basin
	Location of Structure:				
	Downstream Limit/Cros	ss Section:			
	Upstream Limit/Cross S	Section:			
3.	Name of Structure:				
	Type (check one)	☐ Channelization	☐ Bridge/Culvert	☐ Levee/Floodwall	☐ Dam/Basin
	Location of Structure:				
	Downstream Limit/Cros	ss Section:			
	Upstream Limit/Cross S	Section:			
NOT	E: For more structur	res, attach additional	pages as needed.		

B. CHANNELIZATION

, ,,,,,	oding Source:	
Nan	me of Structure:	
1.	Accessory Structures	
	The channelization includes (check one):	
	 □ Levees [Attach Section E (Levee/Floodwall)] □ Superelevated sections □ Debris basin/detention basin [Attach Section D (Dam/Basin)] □ Other (Describe): 	 □ Drop structures □ Transitions in cross sectional geometry □ Energy dissipator
2.	Drawing Checklist	
	Attach the plans of the channelization certified by a registered professional engineer	as described in the instructions.
3.	Hydraulic Considerations	
	The channel was designed to carry (cfs) and/or the -year flood.	
	The design elevation in the channel is based on (check one):	
	☐ Subcritical flow ☐ Critical flow ☐ Supercritical	flow 🗋 Energy grade line
	If there is the potential for a hydraulic jump at the following locations, check all that a controlled without affecting the stability of the channel.	pply and attach an explanation of how the hydraulic jump is
	☐ Inlet to channel ☐ Outlet of channel ☐ At Drop Structures ☐ At Transi☐ Other locations (specify):	tions
4.	Sediment Transport Considerations	
	Was sediment transport considered?	n F (Sediment Transport).
	C. BRIDGE/CULVERT	
Floo		
	oding Source:	
Nan	oding Source: ne of Structure:	
Nan		
Nan	ne of Structure:	
Nan	ne of Structure: 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS	outine, WSPRO, HY8): lysis used for the flooding source could not analyze the
	ne of Structure: 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS Revised analysis of bridge/culvert previously modeled in the FIS 2. Hydraulic model used to analyze the structure (e.g., HEC-2 with special bridge roll different than hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analysis for the flooding source.	lysis used for the flooding source could not analyze the
	ne of Structure: 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS Revised analysis of bridge/culvert previously modeled in the FIS 2. Hydraulic model used to analyze the structure (e.g., HEC-2 with special bridge roll different than hydraulic analysis for the flooding source, justify why the hydraulic analystructures. Attach justification. Attach plans of the structures certified by a registered professional engineer. The plant the information that has been provided): Dimensions (height, width, span, radius, length) Erosion Protect Shape (culverts only) Low Chord Elevent Material Top of Road Elevent Material Top of Road Elevent Material Structure Invertible Material M	ion vations – Upstream and Downstream Elevations – Upstream and Downstream
	ne of Structure: 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS Revised analysis of bridge/culvert previously modeled in the FIS 2. Hydraulic model used to analyze the structure (e.g., HEC-2 with special bridge religible for the flooding source, justify why the hydraulic analysis for the flooding source, justify why the hydraulic analystructures. Attach justification. Attach plans of the structures certified by a registered professional engineer. The plant the information that has been provided): Dimensions (height, width, span, radius, length) Shape (culverts only) Material Beveling or Rounding Wing Wall Angle Structure Invertigation.	ion vations – Upstream and Downstream Elevations – Upstream and Downstream

D. DAM/BASIN

Flo	oding Source:
Naı	ne of Structure:
1.	This request is for (check one):
2.	The dam was designed by (check one): Federal agency State agency Local government agency Private organization
	Name of the agency or organization:
3.	The Dam was permitted as (check one):
	a.
	Provide the permit or identification number (ID) for the dam and the appropriate permitting agency or organization
	Permit or ID number Permitting Agency or Organization
	b.
	Provided related drawings, specification and supporting design information.
4.	Does the project involve revised hydrology? ☐ Yes ☐ No
	If Yes, complete the Riverine Hydrology & Hydraulics Form (Form 2).
	Was the dam/basin designed using critical duration storm?
	Yes, provide supporting documentation with your completed Form 2.
	No, provide a written explanation and justification for not using the critical duration storm.
5.	Does the submittal include debris/sediment yield analysis? ☐ Yes ☐ No
	If yes, then fill out Section F (Sediment Transport). If No, then attach your explanation for why debris/sediment analysis was not considered.
6.	Does the Base Flood Elevation behind the dam or downstream of the dam change?
	Yes No If Yes, complete the Riverine Hydrology & Hydraulics Form (Form 2) and complete the table below.
	Stillwater Elevation Behind the Dam
	FREQUENCY (% annual chance) FIS REVISED
	10-year (10%) 50-year (2%) 100-year (1%) 500-year (0.2%) Normal Pool Elevation
7.	Please attach a copy of the formal Operation and Maintenance Plan

E. LEVEE/FLOODWALL

1.	Sys	stem Elements				
	a.	This Levee/Floodwall analysis is based on (check one):				
	•	upgrading of an existing levee/floodwall system a newly constructed levee/floodwall system reanalysis of an existing levee/floodwall system				
	b.	Levee elements and locations are (check one):	No	Stationing		
	•	structural floodwall State	ation f	to to to		
	C.	Structural Type (check one):				
		 monolithic cast-in place reinforced concrete reinforced concrete masonry block sheet piling Other (describe): 				
	d.	Has this levee/floodwall system been certified by a Federal agency to	o provide pro	otection from the base flood	?	
		☐ Yes No				
		If Yes, by which agency?				
	e.	Attach certified drawings containing the following information (indicate	e drawing sh	neet numbers):		
		Plan of the levee embankment and floodwall structures.	Sheet Nur	mbers:		
		 A profile of the levee/floodwall system showing the Base Flood Elevation (BFE), levee and/or wall crest and foundation, and closure locations for the total levee system. 	Sheet Nur	mbers:		
		A profile of the BFE, closure opening outlet and inlet invert elevations, type and size of opening, and kind of closure.	Sheet Nur	mbers:		
		4. A layout detail for the embankment protection measures.	Sheet Nur	mbers:		
		 Location, layout, and size and shape of the levee embankment features, foundation treatment, floodwall structure, closure structures, and pump stations. 	Sheet Nur	mbers:		
2.	Fre	<u>eeboard</u>				
	a.	The minimum freeboard provided above the BFE is:				
		Riverine				
		3.0 feet or more at the downstream end and throughout3.5 feet or more at the upstream end4.0 feet within 100 feet upstream of all structures and/or constrictions	s No	Structures	Yes Yes	□ No □ No □ No
		Coastal				
		1.0 foot above the height of the one percent wave associated with the stillwater surge elevation or maximum wave runup (whichever is great		ıl-chance	☐ Yes	□ No
		2.0 feet above the 1%-annual-chance stillwater surge elevation			☐ Yes	□ No

E. LEVEE/FLOODWALL (CONTINUED)

3.	Freeboard (continued) Please note, occasionally exceptions are made to the minimum freeboard requirement. If an exception is requested, attach documentation addressing Paragraph 65.10(b)(1)(ii) of the NFIP Regulations. If No is answered to any of the above, please attach an explanation. b. Is there an indication from historical records that ice-jamming can affect the BFE?								
Cha	nnel Station	Left or Righ	nt Bank	Opening	Туре	Highest E	levation for	Type of	Closure Device
							ng Invert		
/F:-	and table on an additi	d abaat	ad and == f :		I				
(EXI	end table on an added	sneet as need	ed and refer	ence)					
4.	In addition to the required detailed analysis reports, data obtained during field and laboratory investigations and used in the design analysis for the following system features should be submitted in a tabulated summary form. (Reference U.S. Army Corps of Engineers [USACE] EM-1110-2-1906 Form 2086.) 4. Embankment Protection a. The maximum levee slope landside is:				U.S. Army				
	Reach	Sideslope	Flow Depth	Velocity	Curve or Straight	D ₁₀₀	Stone Rip	rap Thickness	Depth of Toedown
Sta	to								
Sta	to								
Sta	to								
Sta	to								
Sta	to								 .
Sta	to			_					
	end table on an added	sheet as need	ed and refer	ence each entr	y)		<u>. </u>	<u> </u>	

E. LE	VEE/FL	OODWALL ((CONTINUED)
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4.	Embankment	Protection	(continued)

g. Describe the analysis used for other kinds of protection used (include copies of the design analysis):

Attach engineering analysis to support construction plans.

5. Embankment And Foundation Stability

a. Identify locations and describe the basis for selection of critical location for analysis:

Limiting foundation soil strength:

Section (Ste. , depth 22 to 34 f7. strength
$$\phi = \frac{24-26}{\text{degrees}}$$
, $c = \frac{100-200}{\text{psf}}$ slope: SS = $\frac{1}{100}$ (v)

(Repeat as needed on an added sheet for additional locations)

b. Specify the embankment stability analysis methodology used (e.g., circular arc, sliding block, infinite slope, etc.):

c. Summary of stability analysis results:

Case	Loading Conditions	Critical Safety Factor	Criteria (Min.)
1	End of construction	N/A	1.3
П	Sudden drawdown	1.40	1.0
III	Critical flood stage		1.4
IV	Steady seepage at flood stage	1.33 To 1.48 (See Section 8.5)	1.4
VI	Earthquake (Case I)	1.16 To 1.43	1.0

(Reference: USACE EM-1110-2-1913 Table 6-1)

d. Was a seepage analysis for the embankment performed?

Yes 🗌 No

If Yes, describe methodology used:

finite Element Analysis

e. Was a seepage analysis for the foundation performed?

□ .ss □ .ts

f. Were uplift pressures at the embankment landside toe checked?

Yes 🗌 No

g. Were seepage exit gradients checked for piping potential?

Yes 🗌 No

h. The duration of the base flood hydrograph against the embankment is

hours.

Attach engineering analysis to support construction plans.

E. LEVEE/FLOODWALL (CONTINUED) Floodwall And Foundation Stability a. Describe analysis submittal based on Code (check one): ☐ UBC (1988) or Other (specify): b. Stability analysis submitted provides for: ☐ Overturning ☐ Sliding If not, explain: c. Loading included in the analyses were: ☐ Lateral earth @ P_A = psf; $P_p =$ psf ☐ Surcharge-Slope @ , \square surface psf ☐ Wind @ P_w = psf ☐ Seepage (Uplift); ☐ Earthquake @ P_{eq} = ☐ 1%-annual-chance significant wave height: ft. ☐ 1%-annual-chance significant wave period: sec. d. Summary of Stability Analysis Results: Factors of Safety. Itemize for each range in site layout dimension and loading condition limitation for each respective reach. Criteria (Min) Sta То Sta То **Loading Condition** Overturn Sliding Overturn Sliding Overturn Sliding Dead & Wind 1.5 1.5 Dead & Soil 1.5 1.5 Dead, Soil, Flood, & 1.5 1.5 Impact Dead, Soil, & Seismic 1.3 1.3

(Ref: FEMA 114 Sept 1986; USACE EM 1110-2-2502)

(Note: Extend table on an added sheet as needed and reference)

e. Foundation bearing strength for each soil type:

Bearing Pressure	Sustained Load (psf)	Short Term Load (psf)
Computed design maximum		
Maximum allowable		

f.	Foundation scour protection \square is, \square is not provided. If provided, attach explanation and supporting documentation:
	Attach engineering analysis to support construction plans.

E. LEVEE/FLOODWALL (CONTINUED)

7.	<u>Se</u>	ttlement
	a.	Has anticipated potential settlement been determined and incorporated into the specified construction elevations to maintain the established freeboard margin?
	b.	The computed range of settlement is ft. to ft. N/A Sec Section 8.3
	c.	Settlement of the levee crest is determined to be primarily from :
		Foundation consolidation Embankment compression Other (Describe):
	d.	Differential settlement of floodwalls 🔲 has 🔲 has not been accommodated in the structural design and construction.
		Attach engineering analysis to support construction plans.
8.	Inte	erior Drainage
	a.	Specify size of each interior watershed:
		Draining to pressure conduit: acres Draining to ponding area: acres
	b.	Relationships Established
		Ponding elevation vs. storage
	c.	The river flow duration curve is enclosed:
	d.	Specify the discharge capacity of the head pressure conduit: cfs
	e.	Which flooding conditions were analyzed?
		 Gravity flow (Interior Watershed) Common storm (River Watershed) Historical ponding probability Coastal wave overtopping Yes No No
		If No for any of the above, attach explanation.
	f.	Interior drainage has been analyzed based on joint probability of interior and exterior flooding and the capacities of pumping and outlet facilities to provide the established level of flood protection.
		If No, attach explanation.
	g.	The rate of seepage through the levee system for the base flood is cfs
	h.	The length of levee system used to drive this seepage rate in item g:

E. LEVEE/FLOODWALL (CONTINUED) Interior Drainage (continued) ☐ Yes ☐ No Will pumping plants be used for interior drainage? If Yes, include the number of pumping plants: For each pumping plant, list: Plant #1 Plant #2 The number of pumps The ponding storage capacity The maximum pumping rate The maximum pumping head The pumping starting elevation The pumping stopping elevation Is the discharge facility protected? Is there a flood warning plan? How much time is available between warning and flooding? Will the operation be automatic? ☐ Yes ☐ No If the pumps are electric, are there backup power sources? ☐ Yes ☐ No (Reference: USACE EM-1110-2-3101, 3102, 3103, 3104, and 3105) Include a copy of supporting documentation of data and analysis. Provide a map showing the flooded area and maximum ponding elevations for all interior watersheds that result in flooding. Other Design Criteria a. The following items have been addressed as stated: Liquefaction is is is not a problem Hydrocompaction is is is not a problem Heave differential movement due to soils of high shrink/swell
is is not a problem b. For each of these problems, state the basic facts and corrective action taken: Attach supporting documentation c. If the levee/floodwall is new or enlarged, will the structure adversely impact flood levels and/or flow velocities floodside of the structure? ☐ Yes ☐ No Attach supporting documentation d. Sediment Transport Considerations: Was sediment transport considered? Yes No If Yes, then fill out Section F (Sediment Transport).

If No, then attach your explanation for why sediment transport was not considered.

F. SEDIMENT TRANSPORT
Flooding Source:
Name of Structure:
If there is any indication from historical records that sediment transport (including scour and deposition) can affect the Base Flood Elevation (BFE); and/or based on the stream morphology, vegetative cover, development of the watershed and bank conditions, there is a potential for debris and sediment transport (including scour and deposition) to affect the BFEs, then provide the following information along with the supporting documentation:
Sediment load associated with the base flood discharge: Volume acre-feet
Debris load associated with the base flood discharge: Volume acre-feet
Sediment transport rate (percent concentration by volume)
Method used to estimate sediment transport:
Most sediment transport formulas are intended for a range of hydraulic conditions and sediment sizes; attach a detailed explanation for using the selected method.
Method used to estimate scour and/or deposition:
Method used to revise hydraulic or hydrologic analysis (model) to account for sediment transport: Please note that bulked flows are used to evaluate the performance of a structure during the base flood; however, FEMA does not map BFEs based on bulked flows.
If a sediment analysis has not been performed, an explanation as to why sediment transport (including scour and deposition) will not affect the BFEs or structures must be provided.